



Titre: Discussion of "Consequences of dike breaches and dike overflow in a bifurcating river system" by Anouk Bomers, Ralph M. J. Schielen and Suzanne J. M. H. Hulscher
Title:

Auteurs: Eman AlQasimi, & Tew-Fik Mahdi
Authors:

Date: 2020

Type: Article de revue / Article

Référence: AlQasimi, E., & Mahdi, T.-F. (2020). Discussion of "Consequences of dike breaches and dike overflow in a bifurcating river system" by Anouk Bomers, Ralph M. J. Schielen and Suzanne J. M. H. Hulscher [Commentaire ou lettre]. Natural Hazards, 103 (2), 1629-1632. <https://doi.org/10.1007/s11069-020-03904-1>
Citation:

 **Document en libre accès dans PolyPublie**
Open Access document in PolyPublie

URL de PolyPublie: <https://publications.polymtl.ca/6332/>
PolyPublie URL:

Version: Révisé par les pairs / Refereed

Conditions d'utilisation:
Terms of Use:

 **Document publié chez l'éditeur officiel**
Document issued by the official publisher

Titre de la revue: Natural Hazards (vol. 103, no. 2)
Journal Title:

Maison d'édition: Springer
Publisher:

URL officiel: <https://doi.org/10.1007/s11069-020-03904-1>
Official URL:

Mention légale: This is a post-peer-review, pre-copyedit version of an article published in Natural Hazards (vol. 103, no. 2) . The final authenticated version is available online at:
Legal notice: <https://doi.org/10.1007/s11069-020-03904-1>

1 Discussion of “Consequences of dike breaches and dike overflow in a bifurcating river
2 system” by Anouk Bomers, Ralph M. J. Schielen and Suzanne J. M. H. Hulscher.

3 Natural Hazards (2019) 97:309–334. <https://doi.org/10.1007/s11069-019-03643-y>

4

5 **Eman AlQasimi¹, Tew-Fik Mahdi² Ph.D.**

6 ¹ Department of Civil, Geological and Mining Engineering (CGM), Polytechnique Montreal, C.P.

7 6079, succursale Centre-Ville, Montreal, QC H3C 3A7, Canada. Email:

8 eman.alqasimi@polymtl.ca

9 ² Department of Civil, Geological and Mining Engineering (CGM), Polytechnique Montreal, C.P.

10 6079, succursale Centre-Ville, Montreal, QC H3C 3A7, Canada (Corresponding author). Email:

11 tewfik.mahdi@polymtl.ca

12

13 **Abstract**

14 In this discussion, the authors will point out that even if Bomers et al. (2019) tackle an
15 important problem, ignoring the uncertainties related to the roughness coefficients, Manning
16 coefficients, the downstream boundary and most importantly the errors of the chosen
17 software, HEC-RAS, are serious shortcomings of their study.

18

19 Bomers et al. (2019) present an original contribution “ to study the effect of overland flow
20 patterns on downstream discharge partitioning and flood risk capturing the full dynamics of a
21 river delta (therefore including all possible flow patterns due to multiple dike breaches and
22 backwater effects).”

23 To this end, Bomers et al. (2019) used a 1D-2D coupled hydraulic model, based on the HEC-RAS
24 (v. 5.0.3) software, to simulate the discharge propagation from Andernach, Germany, to the
25 Dutch deltaic area. Then a Monte Carlo analysis is performed, to determine the influence of
26 dike breaches on downstream discharges and flood risk, where “only the parameters that
27 influence dike breach outflow are included as uncertain input parameters. Following the
28 method of Apel et al. (2009) and Vorogushyn et al. (2010), these parameters are:

- 29 – Upstream flood wave in terms of hydrograph shape and peak value
- 30 – Flood waves of the main tributaries dependent on the upstream flood wave
- 31 – Dike breach threshold in terms of critical water level (based on fragility curves) indicating
32 when the dike starts to breach
- 33 – Dike breach formation time
- 34 – Final breach width”

35 In this discussion, the authors will point out that even if Bomers et al. (2019) tackle an
36 important problem, ignoring the uncertainties related to the roughness coefficients, Manning
37 coefficients, the downstream boundary and most importantly the errors of the chosen
38 software, HEC-RAS, are serious shortcomings of their study.

39

40 **Hydraulics Modeling**

41 When performing hydraulics modeling, the choice of the numerical tool is a very important
42 task. In fact, a tool under development should be at least tested and proven to be a good one
43 before been used. Then, during the modeling process, the modeler should use a good modeling
44 domain, well discretize it, impose the right boundary conditions, and perform some numerical

45 tests before using the model. One important test to be done is the choice of the numerical
46 mesh size. Indeed, the numerical solution has to be independent of the mesh size.

47 Bomers et al. (2019) neither justified their choice of HEC-RAS (v. 5.0.3) software to perform the
48 hydraulics modeling nor performed numerical tests of the mesh size reduction impact on the
49 numerical solution. The discussers do not have access to the original model of Bomers et al.
50 (2019), but their experience in using HEC-RAS (v. 5.0.3) showed that this version of the software
51 is not a good one for hydraulics modeling. In fact, for some simulations performed by the
52 authors, using HEC-RAS (v. 5.0.3), it has been noticed that dividing the mesh cells by a factor of
53 two causes the multiplication of the corresponding water depth by a factor of 20. Moreover,
54 the use of a coupled 1D-2D approach is another error source. Bomers et al. (2019) did not
55 justify this choice especially since using a 2D approach for the entire domain would have save
56 time and reduce the coupling errors.

57 Bomers et al. (2019) used the diffusive wave equation instead of the full dynamic wave one to
58 perform their calculations. They justified their choice by “Test runs with both sets of equations
59 were performed. Both runs provided almost the same results, as was also found by Moya
60 Quiroga et al. (2016). The maximum discharge at Lobith deviated only 0.3%, and also no
61 significant deviation in flood extent was found. However, the computation time of the run
62 solving the diffusive wave equations was significantly faster. Therefore, the diffusive wave
63 equations are used to compute the flow characteristics (e.g. water level, flow velocity) at each
64 1D-profile and 2D grid cell.” While dealing with the uncertainties effects on modeling it would
65 have been better to use the dynamic equation. In fact, the tests performed by Bomers et al.
66 (2019) to choose the diffusive wave approximation using HEC-RAS (v. 5.0.3) are misleading, and

67 their results may be explained by what was reported later by the developers of HEC-RAS (v.
68 5.0.3). In fact, in the HEC-RAS release note (USACE, 2019), it is reported that “In the shallow
69 water 2D solver, it was noticed that some simulations had flattened velocity profiles across the
70 direction of flow. Numerical diffusion in the advection terms was identified as the cause of the
71 problem, particularly the scheme used for tracking velocity and velocity interpolation in the
72 middle of cells. The interpolation formula was changed and is now computed using a more
73 compact stencil, resulting in less numerical diffusion and more accurate results.” and it is well
74 mentioned that “In general, the new formulation will have less numerical diffusion, and
75 therefore potentially higher velocities and lower water surface elevations. Previously
76 developed/calibrated models may need to have minor Manning’s n value adjustments
77 (increased Manning’s n values) and/or increased turbulent diffusion coefficient (or turn
78 turbulence on if it was not previously on) in order to reproduce previous version results.”
79 Moreover, several developed software, such as FLDWAV (Fread and Lewis, 1998), SRH-1D
80 (Greimann and Huang, 2018) and MIKE11(DHI, 2009), introduce artificial damping of the inertia
81 terms of the full dynamic wave equation when numerical instabilities occur without using the
82 diffusive wave equation in the whole domain.

83

84 **Downstream boundary condition**

85 Bomers et al. (2019) did not consider the influence of the downstream boundary condition
86 when performing a Monte Carlo analysis, to determine the influence of dike breaches on
87 downstream discharges and flood risk. In fact, it is well known that for subcritical flow the
88 downstream boundary condition influences the solution in the domain and so the uncertainties

89 related to this boundary condition (e.g., Cunge et al., 1980; Chaudhry, 2008, and Szymkiewicz,
90 2010). Choosing a uniform boundary condition is not justified and ignoring its uncertainties
91 effects on the solution is a serious shortcoming of Bomers et al. (2019) study.

92 Moreover, when dyke breaching occurs, the 2D flow modeling needs downstream boundary
93 conditions. Bomers et al. (2019) neither mentioned these boundary conditions in their paper,
94 nor dealt with the associated uncertainties.

95

96 **Manning coefficient**

97 For hydraulics modeling in open channel flow, it is well known that Manning coefficient is
98 among the very important parameters that influence the solution. Bomers et al. (2019) used
99 the diffusive wave equations which leads to wrong results, then their calibration and validation
100 are wrong since one is forcing the wrong results to fit the observed values.

101 Bomers et al. (2019) did the calibration and validation to choose Manning coefficients for their
102 study. Firstly, as it can be seen, from their tables 1 and 2, that for discharge reduction of 7% ,
103 causing a change in the observed water levels ranging from -32 cm to 21 cm, the change in the
104 simulated water levels ranges from -22 cm to 11 cm, for the same adopted values of Manning's
105 coefficients. Note that the downstream part or the reach do not experience a significant change
106 because of the downstream boundary conditions effects. So the uncertainty of Manning's
107 coefficients do impact the solution. One cannot ignore this uncertainty when performing
108 uncertainty analysis on the flow discharge in the domain even if the breach parameters'
109 uncertainty are considered.

110 Secondly, since the available data (water depth and discharge) do not cover the entire
111 numerical domain, the choose of Manning coefficients for the entire domain must be followed
112 by taking account of the associated uncertainty when performing a Monte Carlo analysis to
113 determine the influence of dike breaches on downstream discharges and flood risk. Several
114 researchers analyzed the effect of Manning coefficients' uncertainties for flood inundation
115 modelling (for e.g., Ying, H. and Xiaosheng, Q., 2014, and Bellos et al., 2017)

116

117 **Conclusion**

118 Bomers et al. (2019) are to be commended for their efforts to treat the effect of overland flow
119 patterns on downstream discharge partitioning and flood risk. However, the original paper has
120 the following shortcomings:

- 121 - Using software HEC-RAS (v. 5.0.3), developed by the Hydrologic Engineering Centre (HEC)
122 of the US Army Corps of Engineers, is not the right choice because numerical results of
123 this version of the software are less accurate;
- 124 - Using a coupled approach 1D-2D introduces more modeling errors, especially at rivers'
125 junctions;
- 126 - Ignoring the effects of the downstream boundary condition's uncertainties is incorrect
127 since the downstream boundary, and its uncertainties, influence the solution;
- 128 - Ignoring the effect of Manning coefficients' uncertainties is unjustifiable since these
129 coefficients, and their uncertainties, influence directly the solution.

130 Addressing these points will introduce more clarity to the authors' work, which proposes a
131 promising method for studying the effect of overland flow patterns on downstream discharge
132 partitioning and flood risk.

133

134 **Acknowledgment**

135 This research was supported in part by a National Science and Engineering Research Council
136 (NSERC) Discovery Grant for the corresponding author, application No: RGPIN-2016-06413.

137

138 **References**

139 Bellos V., I. M. Kourtis, A. Moreno-Rodenas, and V. A. Tsihrintzis (2017). Quantifying Roughness
140 Coefficient Uncertainty in Urban Flooding Simulations through a Simplified Methodology.

141 Water 2017, 9(12), 944. DOI.org/10.3390/w9120944

142 Chaudhry M. H. (2008). Open-Channel Flow, 2nd Ed., 2008, Springer Berlin.

143 Cunge J. A., F. M. Holly, and A. Verway (1980) Practical Aspects of Computational River
144 Hydraulics, Pitman Publishing: London.

145 DHI (2009) MIKE 11: A modeling system for rivers and channels. Reference manual. Danish
146 Hydraulic Institute, Horsholm, Denmark.

147 Fread D.L., and J.M. Lewis (1998). NWS FLDWAV model: Theoretical Description and User
148 Documentation, Hydrologic research Laboratory, Office of Hydrology, National Weather
149 Service, NOAA, 335 pp.

150 Greimann B., and V. H. Huang (2018). Sedimentation and River Hydraulics – One Dimension,
151 Version 4.0. Sedimentation and River Hydraulics Group, Technical Service Center. Bureau of
152 Reclamation, Denver, CO.

153 Szymkiewicz R. (2010). Numerical Modeling in Open Channel Hydraulics. Water Science and
154 Technology Library, volume 83, Springer.

155 USACE (2019). HEC-RAS Release notes version 5.0.7, March 2019. Retrieved from [https://www.](https://www.hec.usace.army.mil/software/hec-ras/documentation/HEC-RAS_5.0.7_Release_Notes.pdf)
156 [hec.usace.army.mil/software/hec-ras/documentation/HEC-RAS_5.0.7_Release_Notes.pdf](https://www.hec.usace.army.mil/software/hec-ras/documentation/HEC-RAS_5.0.7_Release_Notes.pdf)

157 Ying H., and Q. Xiaosheng (2014). Uncertainty analysis for flood inundation modelling with a
158 random floodplain roughness field. Journal of Environmental Systems Research Vol.3 (9).
159 DOI.10.1186/2193-2697-3-9.

160

161